CITY OF MERCER ISLAND

DEVELOPMENT SERVICES GROUP

9611 SE 36TH STREET | MERCER ISLAND, WA 98040

PHONE: 206.275.7605 | www.mercergov.org

Inspection Requests: Online: www.MyBuildingPermits.com VM: 206.275.7730



00280

SECTION A: SMALL PROJECT STORMWATER SITE PLAN/REPORT

Narrative and Plan Submittal

<u>Instructions</u>: This is a template for a simplified Stormwater Report. This form or an equivalent must accompany your Building Permit Application if the answer is "Yes" to each statement below. If "No" is the answer to one or more of the statements below, a full Drainage Report is required and the project does not qualify for use of the Small Project Stormwater Site Plan/Report template.

Select "yes" or "no" for each statement below. Answer "yes" if the statement accurately describes your project.

Yes	No	Statement
√		This project disturbs less than 1 acre and is not part of a larger common plan of development.
√		This project converts less than 3/4 acre to lawn or landscape areas.
✓		This project will create, add, or replace (in any combination) 2,000 square feet or greater, but less than 5,000 square feet, of new plus replaced hard surface OR will have a land disturbing activity of 7,000 square feet or greater OR will result in a net increase of impervious surface of 500 square feet or greater.
✓		This project will not adversely impact a wetland, stream, water of the state, or change a natural drainage course.

Basic Project Information

Strand Property	
Project Name:	
Site Address: 6950 SE Maker St.	
Total Lot Size: 8,750	
Total Proposed Area to be Disturbed (including stockpile area): 7,000	sq_ft
Total Volume of Proposed Cut and Fill: 190	sq ft
Total Proposed New Hard Surface Area:	sq ft
Total Proposed Replaced Hard Surface Area: 2,736	sq_ft
Total Proposed Converted Pervious Surface Area (Native vegetation to lawn or landscape):	sq ft
Net Increase in Impervious Surface:	sq ft

Minimum Requirement #1: Preparation of Stormwater Site Plan

Written Project Description:		
See Attached Drainage Report and Downstream A	analysis	
Calculate new or replaced areas by surface type:		
Lawn or Landscape Areas: 3,912 sq ft	Roof Area: 1,808	sq ft
Other Hard Surface Areas:		
Driveway: 1091 sq ft Patio: 91	sq ft Sidewalk: sq	Į ft
Parking Lot: sq ft Other:	sq ft	

✓ Attach Drainage Plan

Drainage Plan shall include the following:

- <u>Scaled drawing</u> with slopes, lot lines, any public-right-of-way and any easements, location of each on-site stormwater management BMP selected above and the areas served by them, buildings, roads, parking lots, driveways, landscape features, and areas of disturbed soils to be amended.
- The scaled drawing must be suitable to serve as a recordable document that will be attached to the property deed for each lot that includes on-site BMPs. Document submittal must follow the "Standard Formatting Requirements for Recording Documents" per King County: www.kingcounty.gov/depts/records-licensing/recorders-office/recording-documents.aspx
- Identify design details and maintenance instructions for each on-site BMP, and attach them to this Small Project Stormwater Site Plan/Report.

MINO	
Min	nimum Requirement #2 : Construction Stormwater Pollution Prevention
\checkmark	Complete Section B of this submittal package: Construction Stormwater Pollution Prevention Plan Narrative (SWPPP
\checkmark	Attach construction SWPPP
Min	nimum Requirement #3 : Source Control of Pollution
availal storm	ection contains practices and procedures to reduce the release of pollutants. Provide a description of all known, ble and reasonable source control BMPs that will be, or are anticipated to be, used at this location to prevent water from coming into contact with pollutants. Additional BMPs are found in Volume IV of the 2014 Stormwater gement Manual for Western Washington (SWMMWW).
Check	the BMPs you will use:
\checkmark	BMP S411 for Landscaping and Lawn/ Vegetation Management Operational practices for sites with landscaping
	BMP S421 for Parking and Storage of Vehicles. Public and commercial parking lots can be sources of suspended solids, metals, or toxic hydrocarbons such oils and greases.
	BMP S433 for Pools, Spas, Hot Tubs, Fountains Discharge from pools, hot tubs, and fountains can degrade ambient water quality. Routine maintenance activities generate a variety of wastes. Direct disposal of these waters to drainage system and waters of the state are not permitted without prior treatment and approval.
	Other BMPs found in Volume IV of SWMMWW applicable to project:

No source control BMPs are applicable for this project.

Minimum Requirement #4: Preservation of Natural Drainage Systems

Natural drainage patterns shall be maintained and discharges from the project site shall occur at the natural location, to the maximum extent practicable. All outfalls require energy dissipation.

Choose the option below that best describes your project:



This site has existing drainage systems or outfalls. These items are shown on the Drainage Plan. Include the following items on the Drainage Plan:

- Pipe invert elevations, slopes, cover, and material
- Locations, grades, and direction of flow in ditches and swales, culverts, and pipes

Describe how these systems will be preserved:

Under existing conditions the site drains to the stormwater system in SE Maker St. This will remain unchanged following site development. See Attached Drainage Report and Downstream Analysis for additional details.				
This site does not have any existing drainage systems or outfalls.				
Additional Comments:				

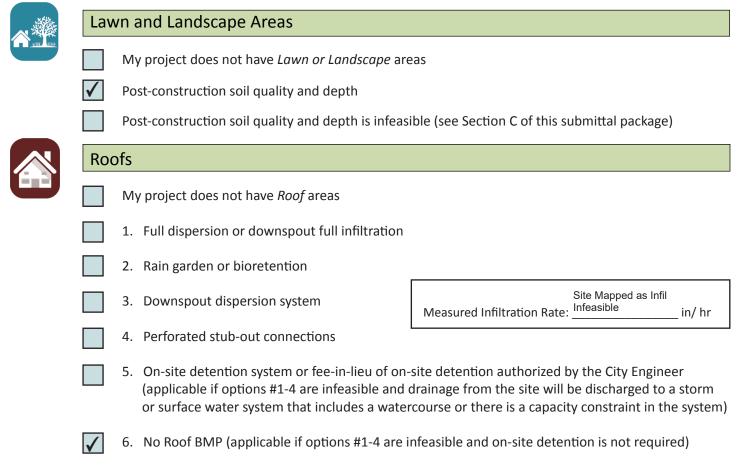
Minimum Requirement #5: On-site Stormwater Management

All projects meeting the thresholds for this Small Project Stormwater Report shall employ on-site stormwater management BMPs (See Small Project Stormwater Requirements Tip Sheet) to infiltrate, disperse, and retain stormwater runoff on-site to the extent feasible without causing flooding or erosion impacts.

List #1

For each category select the <u>first</u> feasible item on the list below. Document your justification for each infeasible BMP in Section C of this submittal package.

Check one option for each category below:



If #5 or #6 is selected, briefly describe why no Roof BMP is feasible (include detailed information in Section C of this submittal package):

The site runoff is reduced following development and the downstream system does not have any capacity constraints prior to discharging to Lake Washington. (see attached drainage report and downstream analysis).

The site is mapped as infiltration infeasible and cannot meet the design requirements for dispersion BMPs.

Minimum Requirement #5: On-site Stormwater Management (cont.) Other Hard Surfaces (such as driveway, sidewalk, parking lot, patio, etc.) My project does not have Other Hard Surface areas Site Mapped as Infil Measured Infiltration Rate: ____ 1. Full dispersion in/ hr 2. Permeable pavement, rain gardens, or bioretention 3. Sheet flow dispersion or concentrated flow dispersion 4. On-site detention system or fee-in-lieu of on-site detention authorized by the City Engineer (applicable if options #1-3 are infeasible and drainage from the site will be discharged to a storm or surface water system that includes a watercourse or there is a capacity constraint in the system) 5. No Other Hard Surface BMP (applicable if options #1-3 are infeasible and on-site detention is not required) If #4 or #5 is selected, briefly describe why no Other Hard Surface BMP is feasible (include detailed information in Section C of this submittal package): The site runoff is reduced following development and the downstream system does not have any capacity constraints prior to discharging to Lake Washington. (see attached drainage report and downstream analysis). The site is mapped as infiltration infeasible and cannot meet the design requirements for dispersion BMPs.

Flow Control Exempt List

Proceed with this list if your project discharges directly to Lake Washington or if findings from a downstream analysis confirm that the downstream system is free of capacity constraints for a minimum of ¼ mile and a maximum of 1 mile.

For flow control exempt discharges, the BMPs listed below for Roofs and Other Hard Surfaces do not need to be evaluated in priority order. You can select any BMP from the lists provided below and do not need to document infeasibility in Section C of this submittal package.



Minimu	um Re	equirement #5 : On-site Stormwater Management (cont.)
	Roo	ofs
		My project does not have <i>Roof</i> areas
		Downspout full infiltration
		Downspout dispersion system
		Perforated stub-out connections
	\checkmark	Each item above is infeasible
If "Each ite	m abov	ve is infeasible" is selected, briefly describe why no Roof BMP is feasible:
The site i	is ma	oped as infiltration infeasible and cannot meet the design requirements for dispersion
Bivii o.		
	Oth	ner Hard Surfaces (such as driveway, sidewalk, parking lot, patio, etc.)
	Oti	iei Haid Surfaces (such as driveway, sidewaik, parking lot, patio, etc.)
		My project does not have Other Hard Surface areas
		Sheet flow dispersion
		Concentrated flow dispersion
	\checkmark	Each item above is infeasible
If "Each ite	m abov	ve is infeasible" is selected, briefly describe why no Other Hard Surface BMP is feasible:
	const	f is reduced following development and the downstream system does not have any traints prior to discharging to Lake Washington. (see attached drainage report and nalysis).
The site	canno	ot meet the design requirements for dispersion BMPs.

Instructions

This is a template for a simplified Construction Stormwater Pollution Prevention Plan ("Construction SWPPP"). If "No" is the answer to one or more of the statements on the first page of Section A of this submittal package, then a full Construction SWPPP is required and the project does not quality for the use of the Small Project Construction SWPPP Narrative template. If the project is less than the thresholds on the first page of Section A of this submittal package, then Minimum Requirement #2 still applies, but this section (Section B) or a full construction SWPPP is not required. You should include your Construction SWPPP in your contract with your builder. A copy of the Construction SWPPP must be located at the construction site or within reasonable access to the site for construction and inspection personnel at all times.

General Information on the Existing Site and Project

Describe the following in the Project Narrative box below (attach additional pages if necessary):

- Nature and purpose of the construction project
- Existing topography, vegetation, and drainage, and building structures
- Adjacent areas, including streams, lakes, wetlands, residential areas, and roads that might be affected by the
 construction project
- How upstream drainage areas may affect the site
- Downstream drainage leading from the site to the receiving body of water
- Areas on or adjacent to the site that are classified as critical areas
- Critical areas that receive runoff from the site up to one-quarter mile away
- Special requirements and provisions for working near or within critical areas
- Areas on the site that have potential erosion problems

See Attached Drainage Report and Downstream Analysis

Project Narrative:

00007

Construction SWPPP Drawings

Locations and outlets of any dewatering systems.

Refer to the general Drawing Requirements in Stormwater Management Manual for Western Washington (SWMMWW) Volume I, Chapter 3.

Vic	cinity Map		
Prov	vide a map with enough detail to identify the location of	the co	onstruction site, adjacent roads, and receiving waters.
Sit	е Мар		
Inclu	ide the following (where applicable):		
\checkmark	Legal description of the property boundaries or an illustration of property lines (including distances) on the drawings.	\checkmark	Final and interim grade contours as appropriate, drainage basins, and the direction of stormwater flow during and upon completion of construction.
√	North arrow.	\checkmark	Areas of soil disturbance, including all areas affected by clearing, grading, and excavation.
✓	Existing structures and roads. Boundaries and identification of different soil types.	\checkmark	Locations where stormwater will discharge to surface waters during and upon completion of construction.
\checkmark	Areas of potential erosion problems.	\checkmark	Existing unique or valuable vegetation and vegetation to be preserved.
	Any on-site and adjacent surface waters, critical areas, buffers, flood plain boundaries, and Shoreline Management boundaries.	\checkmark	Cut-and-fill slopes indicating top and bottom of slope catch lines.
\checkmark	Existing contours and drainage basins and the direction of flow for the different drainage areas.	\checkmark	Total cut-and-fill quantities and the method of disposal for excess material.
√	Where feasible, contours extend a minimum of 25 feet beyond property lines and extend sufficiently to depict existing conditions.	\checkmark	Stockpile; waste storage; and vehicle storage, maintenance, and washdown areas.
Te	mporary and Permanent BMPs		
Inclu	ide the following on site map (where applicable):		
\checkmark	Locations for temporary and permanent swales, interceptor trenches, or ditches.		Details for bypassing off-site runoff around disturbed areas
	Drainage pipes, ditches, or cut-off trenches associated with erosion and sediment control and stormwater management.		Locations of temporary and permanent stormwater treatment and/or flow control best management practices (BMPs).
\checkmark	Temporary and permanent pipe inverts and minimum slopes and cover.	\checkmark	Details for all structural and nonstructural erosion and sediment control (ESC) BMPs (including, but not limited to, silt fences, construction entrances, sedimentation facilities, etc.)
	Grades, dimensions, and direction of flow in all ditches and swales, culverts, and pipes.		Details for any construction-phase BMPs or techniques used for Low Impact Development (LID) BMP protection.

Element 1: Preserve Vegetation / Mark Clearing Limits

The goal of this element is to preserve native vegetation and to clearly show the limits of disturbance.				
This element does not apply to my project because:				
The site was cleared as part of clearing activity that is subject to an enforcement action and is re-vegetated. Restoration may be necessary to comply with Critical Area Regulations or NPDES requirements. Buffer Zones-BMP C102 may apply if Critical Areas exist on-site and buffer zones shall be protected.				
Other Reason / Additional Comments:				
If it <u>does</u> apply, describe the steps you will take and select the best management practices (BMPs) you will use:				
The perimeter of the area to be cleared shall be marked prior to clearing operation with visible flagging, orange plastic barrier fencing and/or orange silt fencing as shown on the SWPPP site map. The total disturbed area shall be less than 7,000 square feet. Vehicles will only be allowed in the areas to be graded, so no compaction of the undeveloped areas will occur.				
Additional Comments:				
Silt Fence will be installed around the site perimeter to function as a hig visibility fence and to provide sediment control. See Sheet C-1.				
Check the BMPs you will use:				
C101 Preserving Natural Vegetation C102 Buffer Zones C103 High Visibility Fence				

Element 2: Construction Access

The goal of this element is	s to provide a stabiliz	ed construction	entrance/exit to	prevent or red	duce or sedim	ent
track out.						

rack o	out.
his ele	ement <u>does not</u> apply to my project because:
\checkmark	The driveway to the construction area already exists and will be used for construction access. All equipment and vehicles will be restricted to staying on that existing impervious surface.
	Other Reason / Additional Comments:
fit <u>do</u>	A stabilized construction entrance will be installed prior to any vehicles entering the site, at the location shown on the SWPPP site map.
Addit	tional Comments:
heck	the BMPs you will use:
	C105 Stabilized Construction C106 Wheel Wash C107 Construction Road / Parking Area Stabilization

Element 3: Control Flow Rates

The goal of this element is to construct retention or detention facilities when necessary to protect properties and waterways downstream of development sites from erosion and turbid discharges.

This element **does not** apply to my project because:

\checkmark	Other Reason / Additional Comments:
Give	n the size of the development the use of the BMPs to control runoff flow rates is not warranted.
f it doe	es apply, describe the steps you will take and select the BMPs you will use:
	Flow rates will be controlled by using SWPPP Element 4 sediment controls and BMP T5.13 Post-Construction Soil Quality and Depth if necessary.
Additi	ional Comments:

Element 4: Sediment Control

The goal of this element	t is to construc	t sediment contr	ol BMPs that	: minimize s	sediment d	discharges	from the
site.							

This element <u>does not</u> apply to my project because:	
The site has already been stabilized and re-vegetated.	
Other Reason / Additional Comments:	
If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:	
Sediment control BMPs shall be placed at the locations shown on the SWPPP site map	
Additional Comments:	
Silt Fonce will be installed around the site perimeter to function as a big visibility fonce and to	
Silt Fence will be installed around the site perimeter to function as a hig visibility fence and to provide sediment control. See Sheet C-1.	
provide sediment control. See Sheet C-1.	

Element 5: Stabilize Soils
The goal of this element is to stabilize exposed and unworked soils by implementing erosion control BMPs.
This element <u>does not</u> apply to my project because:
Other Reason / Additional Comments:
f it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:
Exposed soils shall be worked during the week until they have been stabilized. Soil stockpiles will be located within the disturbed area shown on the SWPPP site map. Soil excavated for the foundation will be backfilled against the foundation and graded to drain away from the building. No soils shall remain exposed and unworked for more than 7 days from May 1 to September 30 or more than 2 days from October 1 to April 30. Once the disturbed landscape areas are graded, the grass areas will be amended using BMP T5.13 Post-Construction Soil Quality and Depth. All stockpiles will be covered with plastic or burlap if left unworked. Additional Comments:
See TESC Plan C-1 and C-2 for soil ammendment
Check the BMPs you will use:
C120 Temporary & C122 Nets & Blankets C124 Sodding C131 Gradient C235 Wattle Permanent Seeding C130 Temporary & C130 Temporar
C121 Mulching C123 Plastic Covering C125 Topsoil / Composting C140 Dust Control

Element 6: Protect Slopes

The goal	of this	element	is to desi	gn and	construct	cut-and-fill	slopes in	n a mannei	r to minimize	erosion.
	0			O			0.000			

The goal of this element is to design and construct cut-and-fill slopes in a manner to minimize erosion.					
This ele	ement <mark>does not</mark> apply to my projec	t becau	se:		
\checkmark	No cut slopes over 4 feet high or 4 feet high will exceed 3 feet hor engineered slope protection.				foot vertical, and no fill slopes over is no requirement for additional
	Other Reason / Additional Comn	nents:			
If it do	oes apply, describe the steps you w	vill take	and select the BMPs you w	ill use:	
Additi	ional Comments:				
Check	the BMPs you will use:				
	C120 Temporary & Permanent Seeding		C205 Subsurface Drains		C207 Check Dams
	C204 Pipe Slope Drains		C206 Level Spreader		C208 Triangular Silt Dike (Geotextile-Encased Check Dam)

Element 7: Protect Permanent Drain Inlets

The goal of this element is to protect storm drain inlets during construction to prevent stormwater runoff from entering the conveyance system without being filtered or treated.

This elen	ment <u>does not</u> apply to my project because:
Т	he site has open ditches in the right-of-way or private road right-of-way.
Т	here are no catch basins on or near the site.
	Other Reason / Additional Comments:
If it doe	es apply, describe the steps you will take and select the BMPs you will use:
	ratch basins on the site or immediately off site in the right-of-way are shown on the SWPPP site map. Storm rain inlet protection shall be installed.
Additio	onal Comments:
Inlet p	protection will be provided at both catch basins located in the adjacent right-of-way.

Check the BMPs you will use:



C220 Storm Drain Inlet Protection

Element 8: Stabilize Channels and Outlets

The goal of this element is to design, construct, and stabilize on-site conveyance channels to prevent erosion from entering existing stormwater outfalls and conveyance systems.

This element does not apply to my project because:
Construction will occur during the dry weather. No storm drainage channels or ditches shall be constructed either temporary or permanent. A small swale shall be graded to convey yard drainage around the structure using a shallow slope; it shall be seeded after grading and stabilized.
Other Reason / Additional Comments:
If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:
A wattle shall be placed at the end of the swale to prevent erosion at the outlet of the swale.
Additional Comments:
Check the BMPs you will use:
C202 Channel Lining C207 Check Dams C209 Outlet Protection C235 Wattles

F	lement	9:	Contro	l Pol	lutants

The goal of this element is to design, install, implement and maintain BMPs to minimize the discharge of	
pollutants from material storage areas, fuel handling, equipment cleaning, management of waste materials,	etc.

_	tants from material storage areas, fuel handling, equi	pment cleaning, management of waste materials, etc
This e	lement does not apply to my project because:	
	Other Reason / Additional Comments:	
√	Any and all pollutants, chemicals, liquid products and ot human health or the environment will be covered, contashall be kept under cover in a secure location on-site. Co	her materials that have the potential to pose a threat to nined, and protected from vandalism. All such products
Addi	tional Comments:	,
Ched	ck the BMPs you will use:	
\checkmark	C151 Concrete Handling	C152 Sawcutting and Surfacing Pollution Prevention
1	C153 Material Delivery, Storage, and Containment	C154 Concrete Washout Area

Element 10: Control De-watering
The goal of this element is to handle turbid or contaminated dewatering water separately from stormwater.
This element <u>does not</u> apply to my project because:
No dewatering of the site is anticipated.
Other Reason / Additional Comments:
If it <u>does</u> apply, describe the steps you will take and select the BMPs you will use:
Additional Comments:
Check the BMPs you will use:
C203 Water Bars C236 Vegetated Filtration C206 Level Spreader

Element 11: Maintain Best Management Practices

The goal of this element is to maintain and repair all temporary and permanent erosion and sediment control BMPs to assure continued performance.

Describe the steps you will take:



Best Management Practices or BMPs shall be inspected and maintained during construction and removed within 30 days after the City Inspector or Engineer determines that the site is stabilized, provided that they may be removed when they are no longer needed.

Element 12: Manage the Project

The goal of this element is to ensure that the construction SWPPP is properly coordinated and that all BMPs are deployed at the proper time to achieve full compliance with City regulations throughout the project.

If it **does** apply, describe the steps you will take and select the BMPs you will use:

The Construction SWPPP will be implemented at all times. The applicable erosion control BMPs will be implemented in the following sequence:

the f	following sequence:
\checkmark	1. Mark clearing limits
	2. Install stabilized construction entrance
\checkmark	3. Install protection for existing drainage systems and permanent drain inlets
\checkmark	4. Establish staging areas for storage and handling polluted material and BMPs
\checkmark	5. Install sediment control BMPs
\checkmark	6. Grade and install stabilization measures for disturbed areas
\checkmark	7. Maintain BMPs until site stabilization, at which time they may be removed
Addi	tional Comments:

Element 13: Protect Low Impact Development BMPs

The goal of this element is to protect on-site stormwater management BMPs (also known as "Low Impact Development BMPs") from siltation and compaction during construction. On-site stormwater management BMPs used for runoff from roofs and other hard surfaces include: full dispersion, roof downspout full infiltration or dispersion systems, perforated stubout connections, rain gardens, bioretention systems, permeable pavement, sheetflow dispersion, and concentrated flow dispersion. Methods for protecting on-site stormwater management BMPs include sequencing the construction to install these BMPs at the latter part of the construction grading operations, excluding equipment from the BMPs and the associated areas, and using the erosion and sedimentation control BMPs listed below.

the erosion and sedime	ntation control BMPs listed below.		, .
Describe the construction sequencing you will use:			
Additional Comments:			
On-site BMPs are no	t feasible for this project.		
Select the BMPs you will u	ise:		
C102 Buffer Zone	C103 High Visibility Fence	C231 Brush Barrier	
C233 Silt Fence	C234 Vegetated Strip		

Minimum Requirement #5 (On-Site Stormwater Management)

The following tables summarize infeasibility criteria that can be used to justify not using various on-site stormwater management best management practices (BMPs) for consideration for Minimum Requirement #5. This information is also included under the detailed descriptions of each BMP in the 2014 Stormwater Management Manual for Western Washington (Stormwater Manual), but is provided here in this worksheet for additional clarity and efficiency. Where any inconsistencies or lack of clarity exists, the requirements in the main text of the Stormwater Manual shall be applied. If a project is limited by one or more of the infeasibility criteria specified below, but an applicant is interested in implementing a specific BMP, a functionally equivalent design may be submitted to the City for review and approval. Evaluate the feasibility of the BMPs in priority order based on List #1 or #2 (Small Project Stormwater Requirements Tip Sheet and Stormwater Manual). Select the first BMP that is considered feasible for each surface type. Document the infeasibility (narrative description and rationale) for each BMP that was not selected. Only one infeasibility criterion needs to be selected for a BMP before evaluating the next BMP on the list. Attach additional pages for supporting information if necessary.

Note: If your project discharges directly to Lake Washington (flow control exempt) or a downstream analysis confirms that the downstream system is free of capacity constraints for a minimum of ¼ mile and a maximum of 1 mile, then you do not need to complete this worksheet, but should still refer to the infeasibility criteria when selecting BMPs.

	Lawn and Landscaped Areas			
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected		
Post-construction Soil Quality and Depth List #1 and #2	Siting and design criteria provided in BMP T5.13 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. Lawn and landscape area is on till slopes greater than 33 percent.			
	Roofs			
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected		
Full Dispersion List #1 and #2	Site setbacks and design criteria provided in BMP T5.30 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A 65 to 10 ratio of forested or native vegetation area to impervious area cannot be achieved. A minimum forested or native vegetation flowpath length of 100 feet (25 feet for sheet flow from a non-native pervious surface) cannot be achieved.	No native veg. is located on-site and the developed impervious area is 40%.		
Downspout Full Infiltration List #1 and #2	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards. Site setbacks and design criteria provided in BMP T5.10A (Stormwater Manual Volume III, Section 3.1.1) cannot be achieved. The lot(s) or site does not have out-wash or loam soils. There is not at least 3 feet or more of permeable soil from the proposed final grade to the seasonal high groundwater table or other impermeable layer. There is not at least 1 foot or more of permeable soil from the proposed bottom of the infiltration system to the seasonal high groundwater table or other impermeable layer.	Site is mapped as infeasible for infiltration.		

	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	Note: Criteria with setback distances are as measured from the bottom edge of the bioretention soil mix.	
	Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist):	
	Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or down-gradient flooding.	
	Within an area whose ground water drains into an erosion hazard, or landslide hazard area.	
Bioretention or Rain Gardens	Where the only area available for siting would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, pre-existing structures, or pre-existing road or parking lot surfaces.	
List #1 (both) and List #2 (bioretention only)	Where the only area available for siting does not allow for a safe overflow pathway to stormwater drainage system or private storm sewer system.	
	Where there is a lack of usable space for bioretention areas at redevelopment sites, or where there is insufficient space within the existing public right-of-way on public road projects.	
	Where infiltrating water would threaten existing below grade basements.	
	Where infiltrating water would threaten shoreline structures such as bulkheads.	
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation):	
	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards	
	Within setback provided for BMP T7.30 (Stormwater Manual Volume V, Section 7.4)	
	Where they are not compatible with surrounding drainage system as determined by the city (e.g., project drains to an existing stormwater collection system whose elevation or location precludes connection to a properly functioning bioretention area).	00302

	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Applicable	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where land for bioretention is within an erosion hazard, or landslide hazard area (as defined by MICC 19.07.060). Where the site cannot be reasonably designed to locate bioretention areas on slopes less than 8 percent. Within 50 feet from the top of slopes that are greater than 20 percent and over 10 feet of vertical relief. For properties with known soil or groundwater contamination (typically federal Superfund sites or state cleanup sites under the Model Toxics Control Act [MTCA]): Within 100 feet of an area known to have deep soil contamination. Where groundwater modeling indicates infiltration will likely increase or change the direction of the migration of pollutants in the groundwater. Wherever surface soils have been found to be contaminated unless those soils are removed within 10 horizontal feet from the infiltration area. Any area where these facilities are prohibited by an approved cleanup plan under the state MTCA or Federal Superfund Law, or an environmental covenant under Chapter 64.70 RCW. Within 100 feet of a closed or active landfill. Within 10 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is 1,100 gallons or less. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface. Within 100 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is greater than 1,100 gallons.	and Rationale for Each
		00000

	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Bioretention or Rain Gardens (cont.)	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where field testing indicates potential bioretention/rain garden sites have a measured (a.k.a., initial) native soil saturated hydraulic conductivity less than 0.30 inches per hour. A small-scale or large-scale PIT in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to demonstrate infeasibility of bioretention areas. If the measured native soil infiltration rate is less than 0.30 in/hour, bioretention/rain garden BMPs are not required to be evaluated as an option in List #1 or List #2. In these slow draining soils, a bioretention area with an underdrain may be used to treat pollution-generating surfaces to help meet Minimum Requirement #6, Runoff Treatment. If the underdrain is elevated within a base course of gravel, it will also provide some modest flow reduction benefit that will help achieve Minimum Requirement #7. Where the minimum vertical separation of 3 feet to the seasonal high groundwater elevation or other impermeable layer would not be achieved below bioretention that would serve a drainage area that exceeds the following thresholds (and cannot reasonably be broken down into amounts smaller than indicated): o 5,000 square feet of pollution-generating impervious surface (PGIS) o 10,000 square feet of impervious area o 0.75 acres of lawn and landscape. Where the minimum vertical separation of 1 foot to the seasonal high groundwater or other impermeable layer would not be achieved below bioretention that would serve a drainage area less than the above thresholds. Within 100 feet of a drinking water well, or a spring used for drinking water supply. Within 10 feet of small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks from a "large on-site sewage disposal system," see Chapter 246-272B WAC.	BMP Not Selected
		00204

	Roofs (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Downspout Dispersion Systems List #1 and #2	Site setbacks and design criteria provided in BMP T5.10B (Stormwater Manual Volume III, Section 3.1.2) cannot be achieved. For splash blocks, a vegetated flowpath at least 50 feet in length from the downspout to the downstream property line, structure, stream, wetland, slope over 15 percent, or other impervious surface is not feasible. For trenches, a vegetated flowpath of at least 25 feet in between the outlet of the trench and any property line, structure, stream, wetland, or impervious surface is not feasible. A vegetated flowpath of at least 50 feet between the outlet of the trench and any slope steeper than 15 percent is not feasible.	Flow path length and slope requirements cannot be met for dispersion BMPs.
	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards	Site is mapped as infeasible for infiltration.
Perforated Stub-Out Connections List #1 and #2	For sites with septic systems, the only location available for the perforated portion of the pipe is located up-gradient of the drainfield primary and reserve areas. This requirement can be waived if site topography will clearly prohibit flows from intersecting the drainfield or where site conditions (soil permeability, distance between systems, etc.) indicate that this is unnecessary.	
	Site setbacks and design criteria provided in BMP T5.10C (Stormwater Manual Volume III, Section 3.1.3) cannot be achieved.	
	There is not at least 1 foot of permeable soil from the proposed bottom (final grade) of the perforated stub-out connection trench to the highest estimated groundwater table or other impermeable layer.	
	The only location available for the perforated stub-out connection is under impervious or heavily compacted soils.	
On-site Detention List #1 and #2	Project discharges directly to Lake Washington. Findings from a 1/4 mile downstream analysis confirm that the downstream system is free of capacity constraints. Site setbacks and design criteria provided in the Stormwater Manual (Volume III, Section 3.2.2) cannot be achieved.	Site drains to closed conveyance system with adequate capacity. See attached downstream analysis.
		00305

	Other Hard Surfaces	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
Full Dispersion List #1 and #2	Site setbacks and design criteria provided in BMP T5.30 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A 65 to 10 ratio of forested or native vegetation area to impervious area cannot be achieved. A minimum forested or native vegetation flowpath length of 100 feet (25 feet for sheet flow from a non-native pervious surface) cannot be achieved.	No native veg. is located on-site and the developed impervious area is 40%.
Permeable Pavement List #1 and #2	Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist): Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or downgradient flooding. Within an area whose ground water drains into an erosion hazard, or landslide hazard area. Where infiltrating and ponded water below the new permeable pavement area would compromise adjacent impervious pavements. Where infiltrating water below a new permeable pavement area would threaten existing below grade basements. Where infiltrating water would threaten shoreline structures such as bulkheads. Down slope of steep, erosion prone areas that are likely to deliver sediment. Where fill soils are used that can become unstable when saturated. Excessively steep slopes where water within the aggregate base layer or at the subgrade surface cannot be controlled by detention structures and may cause erosion and structural failure, or where	
	 surface runoff velocities may preclude adequate infiltration at the pavement surface. Where permeable pavements cannot provide sufficient strength to support heavy loads at industrial facilities such as ports. Where installation of permeable pavement would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, or pre-existing road subgrades. 	00000

	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	further justification (though some require professional services to make the	Site is mapped as infeasible for infiltration.
	Evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards	
	Within an area designated as an erosion hazard, or landslide hazard.	
	Within 50 feet from the top of slopes that are greater than 20 percent.	
	For properties with known soil or groundwater contamination (typically federal Superfund sites or state cleanup sites under MTCA):	
	 Within 100 feet of an area known to have deep soil contamination. 	
Permeable Pavement (cont.)	 Where groundwater modeling indicates infiltration will likely increase or change the direction of the migration of pollutants in the groundwater. 	
(cont.)	 Wherever surface soils have been found to be contaminated unless those soils are removed within 10 horizontal feet from the infiltration area. 	
	 Any area where these facilities are prohibited by an approved cleanup plan under the state MTCA or Federal Superfund Law, or an environmental covenant under Chapter 64.70 RCW. 	
	Within 100 feet of a closed or active landfill.	
	Within 100 feet of a drinking water well, or a spring used for drinking water supply, if the pavement is a pollution-generating surface.	
	Within 10 feet of a small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks from a "large on-site sewage disposal system," see Chapter 246-272B WAC.	
	Within 10 feet of any underground storage tank and connecting underground pipes, regardless of tank size. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface.	
	At multi-level parking garages, and over culverts and bridges.	
	Where the site design cannot avoid putting pavement in areas likely to have long-term excessive sediment deposition after construction (e.g., construction and landscaping material yards).	

Other Hard Surfaces (cont.)			
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected	
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where the site cannot reasonably be designed to have: • Porous asphalt surface < 5% slope • Pervious concrete surface < 10% slope • Permeable interlocking concrete pavement surface < 12% slope • Grid systems < 6-12% slope (check with manufacturer and local supplier to confirm maximum slope) Where the subgrade soils below a pollution-generating permeable pavement (e.g., road or parking lot) do not meet the soil suitability criteria for providing treatment. See soil suitability criteria for treatment in the Stormwater Manual Volume III, Section 3.3.7. Note: In these instances, the city may approve installation of a 6 inch sand filter layer meeting city specifications for treatment as a condition of construction. Where underlying soils are unsuitable for supporting traffic loads when saturated. Soils meeting a California Bearing Ratio of 5 percent are considered suitable for residential access roads. Where replacing existing impervious surfaces unless the existing surface is a non-pollution generating surface over an outwash soil with a saturated hydraulic conductivity of 4 inches per hour or greater. Where appropriate field testing indicates soils have a measured (a.k.a., initial) subgrade soil saturated hydraulic conductivity less than 0.3 inches per hour. Only small-scale PIT or large-scale PIT methods in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to evaluate infeasibility of permeable pavement areas. (Note: In these instances, unless other infeasibility restrictions apply, roads and parking lots may be built with an underdrain, preferably elevated within the base course, if flow control benefits are desired.) Roads that receive more than very low traffic volumes, and areas having more than very low truck traffic volumes, and areas having more than ver		
		00308	

	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): At sites defined as "high-use sites" (refer to the Glossary in the	
Permeable Pavement (cont.)	Stormwater Manual Volume I). In areas with "industrial activity" as identified in 40 CFR 122.26(b)(14).	
	Where the risk of concentrated pollutant spills is more likely such as gas stations, truck stops, and industrial chemical storage sites.	
	Where routine, heavy applications of sand occur in frequent snow zones to maintain traction during weeks of snow and ice accumulation.	
	Where the seasonal high groundwater or an underlying impermeable/ low permeable layer would create saturated conditions within 1 foot of the bottom of the lowest gravel base course.	
	Note: Criteria with setback distances are as measured from the bottom edge of the bioretention soil mix.	
	Citation of any of the following infeasibility criteria must be based on an evaluation of site-specific conditions and a written recommendation from an appropriate licensed professional (e.g., engineer, geologist, hydrogeologist):	
Bioretention or Rain Gardens	Where professional geotechnical evaluation recommends infiltration not be used due to reasonable concerns about erosion, slope failure, or down-gradient flooding.	
List #1 (both) and List #2 (bioretention	Within an area whose ground water drains into an erosion hazard, or landslide hazard area.	
only)	Where the only area available for siting would threaten the safety or reliability of pre-existing underground utilities, pre-existing underground storage tanks, pre-existing structures, or pre-existing road or parking lot surfaces.	
	Where the only area available for siting does not allow for a safe overflow pathway to stormwater drainage system or private storm sewer system.	
	Where there is a lack of usable space for bioretention areas at redevelopment sites, or where there is insufficient space within the existing public right-of-way on public road projects.	
	Where infiltrating water would threaten existing below grade basements.	
	Where infiltrating water would threaten shoreline structures such as bulkheads.	

	Other Hard Surfaces (cont.)	
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected
	The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Where evaluation of infiltration is not required per the Infiltration Infeasibility Map due to steep slopes, erosion hazards, or landslide hazards.	Site is mapped as infeasible for infiltration. See attached figure.
	Within setback provided for BMP T7.30 (Stormwater Manual Volume V, Section 7.4)	
	Where they are not compatible with surrounding drainage system as determined by the city (e.g., project drains to an existing stormwater collection system whose elevation or location precludes connection to a properly functioning bioretention area).	
Bioretention or	Where land for bioretention is within an erosion hazard, or landslide hazard area (as defined by MICC 19.07.060).	
Rain Gardens (cont.)	Where the site cannot be reasonably designed to locate bioretention areas on slopes less than 8 percent.	
	Within 50 feet from the top of slopes that are greater than 20 percent and over 10 feet of vertical relief.	
	For properties with known soil or groundwater contamination (typically federal Superfund sites or state cleanup sites under the Model Toxics Control Act [MTCA]):	
	Within 100 feet of an area known to have deep soil contamination.	
	 Where groundwater modeling indicates infiltration will likely increase or change the direction of the migration of pollutants in the groundwater. 	
	 Wherever surface soils have been found to be contaminated unless those soils are removed within 10 horizontal feet from the infiltration area. 	
	 Any area where these facilities are prohibited by an approved cleanup plan under the state MTCA or Federal Superfund Law, or an environmental covenant under Chapter 64.70 RCW. 	
	Within 100 feet of a closed or active landfill.	
	Within 10 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is 1,100 gallons or less. As used in these criteria, an underground storage tank means any tank used to store petroleum products, chemicals, or liquid hazardous wastes of which 10 percent or more of the storage volume (including volume in the connecting piping system) is beneath the ground surface.	

	Infeasibility Criteria The following criteria can be cited as reasons for infeasibility without further justification (though some require professional services to make the observation): Within 100 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is	Infeasibility Description and Rationale for Each BMP Not Selected
	further justification (though some require professional services to make the observation): Within 100 feet of an underground storage tank and connecting underground pipes when the capacity of the tank and pipe system is	
l '	underground pipes when the capacity of the tank and pipe system is	
	greater than 1,100 gallons.	
Bioretention or Rain Gardens (cont.)	Where field testing indicates potential bioretention/rain garden sites have a measured (a.k.a., initial) native soil saturated hydraulic conductivity less than 0.30 inches per hour. A small-scale or large-scale PIT in accordance with Stormwater Manual Volume III, Section 3.3.6 (or an alternative small scale test specified by the City) shall be used to demonstrate infeasibility of bioretention areas. If the measured native soil infiltration rate is less than 0.30 in/hour, bioretention/rain garden BMPs are not required to be evaluated as an option in List #1 or List #2. In these slow draining soils, a bioretention area with an underdrain may be used to treat pollution-generating surfaces to help meet Minimum Requirement #6, Runoff Treatment. If the underdrain is elevated within a base course of gravel, it will also provide some modest flow reduction benefit that will help achieve Minimum Requirement #7. Where the minimum vertical separation of 3 feet to the seasonal high groundwater elevation or other impermeable layer would not be achieved below bioretention that would serve a drainage area that exceeds the following thresholds (and cannot reasonably be broken down into amounts smaller than indicated): o 5,000 square feet of pollution-generating impervious surface (PGIS) o 10,000 square feet of impervious area o 0.75 acres of lawn and landscape. Where the minimum vertical separation of 1 foot to the seasonal high groundwater or other impermeable layer would not be achieved below bioretention that would serve a drainage area less than the above thresholds Within 100 feet of a drinking water well, or a spring used for drinking water supply. Within 10 feet of small on-site sewage disposal drainfield, including reserve areas, and grey water reuse systems. For setbacks from a "large on-site sewage disposal system," see Chapter 246-272B WAC.	
		00244

Other Hard Surfaces (cont.)							
BMP and Applicable Lists	Infeasibility Criteria	Infeasibility Description and Rationale for Each BMP Not Selected					
Sheet Flow	Site setbacks and design criteria provided in BMP T5.12 (Stormwater Manual Volume V, Section 5.3) cannot be achieved.	Flow Path and slope requirements cannot be met for dispersion					
Dispersion	Positive drainage for sheet flow runoff cannot be achieved.	BMPs.					
List #1 and #2	Area to be dispersed (e.g., driveway, patio) cannot be graded to have less than a 15 percent slope.						
	For flat to moderately sloped areas, at least a 10 foot-wide vegetation buffer for dispersion of the adjacent 20 feet of contributing surface cannot be achieved. For variably sloped areas, at least a 25 foot vegetated flowpath between berms cannot be achieved.						
Concentrated Flow Dispersion List #1 and #2	Site setbacks and design criteria provided in BMP T5.11 (Stormwater Manual Volume V, Section 5.3) cannot be achieved. A minimum 3 foot length of rock pad and 50 foot flowpath OR a dispersion trench and 25 foot flowpath for every 700 square feet of drainage area followed with applicable setbacks cannot be achieved. More than 700 square feet drainage area drains to any dispersion device.	Flow Path and slope requirements cannot be met for dispersion BMPs.					
On-site Detention List #1 and #2	Project discharges directly to Lake Washington. Findings from a 1/4 mile downstream analysis confirm that the downstream system is free of capacity constraints. Site setbacks and design criteria provided in the Stormwater Manual (Volume III, Section 3.2.2) cannot be achieved.	Site drains to closed conveyance system with adequate capacity. See attached downstream analysis.					

Attachments Required (Check off required items that are attached)					
Site Plan showing, to scale:					
Areas of undisturbed native vegetation (no amendment required)					
New planting beds (amendment required)					
✓ New turf areas (amendment required)					
✓ Type of soil improvement proposed for each area					
Soil test results (required if proposing custom amendment rates)					
Product test results for proposed amendments					

Total Amendment / Topsoil / Mulch for All Areas

Calculate the quantities needed for the entire site based on all of the areas identified on the Site Plan and the calculations on the following page(s):

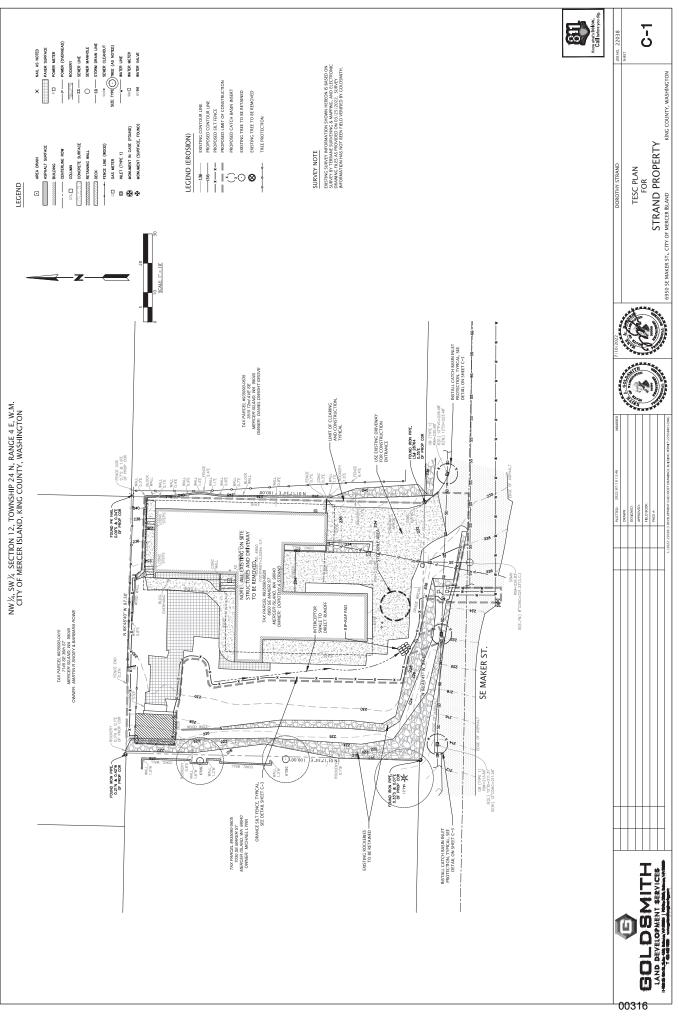
Product	Total Quantity (CY)	Test Results
Product #1:	21CY	% organic matter C:N ratio "Stable"? yes no
Product #2:	CY	% organic matter C:N ratio "Stable"? yes no
Product #3:	CY	% organic matter C:N ratio "Stable"? yes no

Ame	endment / To	opsoil / Mulch by Area	
For each	n identified area	on your Site Plan, provide the following information:	(Use additional sheets if necessary)
Area #	3,912	(should match identified Area # on Site Plan)	
Planting		Undisturbed native vegetation Planting Beds Other:	
Pre-A	Approved Ar	mendment Method	
<i>F</i>	Amend with compost	Turf: SF x 5.4 CY ÷ 1,000 SF = CY Planting beds: SF x 9.3 CY ÷ 1,000 SF= CY Total Quantity = CY Scarification depth: 8 inches	Product:
√ 5	Stockpile and amend	Turf: $3,912$ \blacksquare SF x 5.4 CY \div 1,000 SF = 21 CY Planting beds: SF x 9.3 CY \div 1,000 SF= CY Total Quantity = CY Scarification depth: 8 inches	21 Product:
	Горsoil import	Turf: SF x 18.6 CY÷1,000 SF = CY Planting beds: SF x 18.6 CY ÷ 1,000 SF= CY Total Quantity = CY Scarification depth: 6 inches	Product:
Custo	om Amendn	nent	
<i>F</i>	Amend with compost	Attach information on bulk density, percent organic matter, moisture content, C:N ratio, and heavy metals analysis to support custom amendment rate and scarification depth. Total Quantity =CY Scarification depth:inches	Product:
S	Stockpile and amend	Attach information on bulk density, percent organic matter, moisture content, C:N ratio, and heavy metals analysis to support custom amendment rate and scarification depth. Total Quantity =CY Scarification depth:inches	Product:
Mulc	ch		1
A A	Amend with compost	Planting beds: SF x 12.4 CY ÷ 1,000 SF= CY Total Quantity = CY	Product:
	Stockpile and amend	Planting beds: SF x 12.4 CY ÷ 1,000 SF= CY Total Quantity = CY	Product:
7	Горsoil import	Planting beds: SF x 12.4 CY ÷ 1,000 SF=CY	Product:

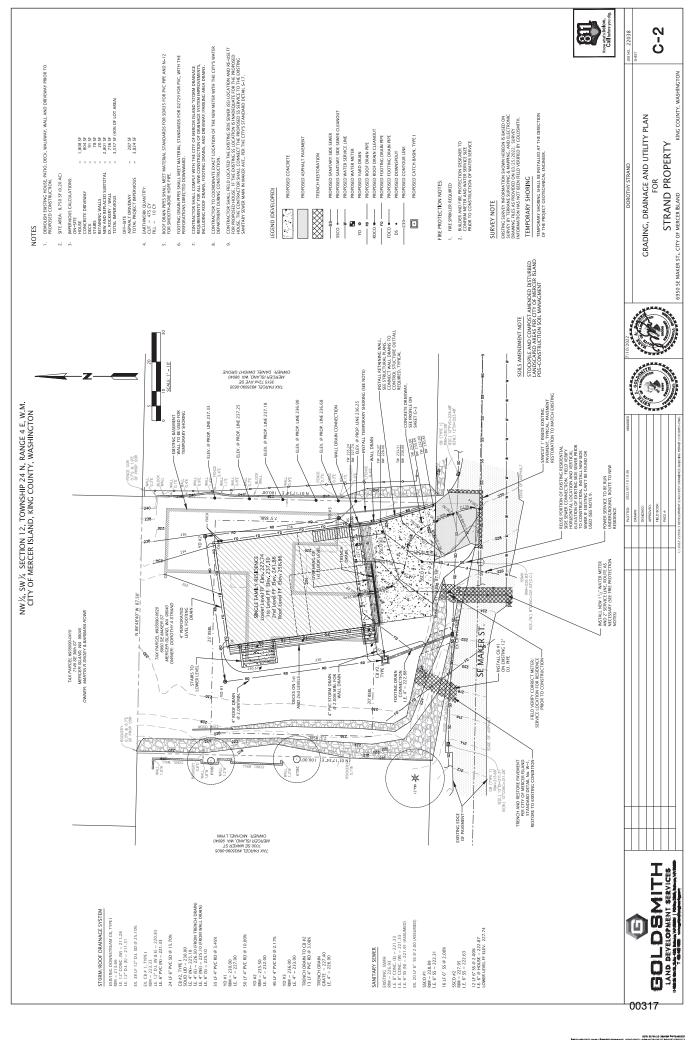
Project Engineer's Certification for Section B

For Stormwater Site Plans with engineered elements, the Construction SWPPP is stamped by a professional engineer licensed in the State of Washington in civil engineering.

J	O	O					
If required, attach a page with the	oroject engineer'	s seal w	ith the following statement:				
			6950 S	E Maker St.			
"I hereby state that this Construction	on Stormwater Po	ollution	Prevention Plan for ———	(name of project)			
has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and							
customary in this community for professional engineers. I understand that the City of Mercer Island does not and will							
not assume liability for the sufficiency, suitability, or performance of Construction SWPPP BMPs prepared by me."							
Applicant Signature for Full Stormwater Package (Sections A through D)							
I have read and completed the Stormwater Submittal Package and know the information provided to be true and correct.							
	Mark Barber F	P.E.					
Print Applicant Name:							
Applicant Signature:	John John John John John John John John		Digitally signed by Mark Barber Date: 2022.07.10 14:23:46-07'00	07/10/2022 Date			

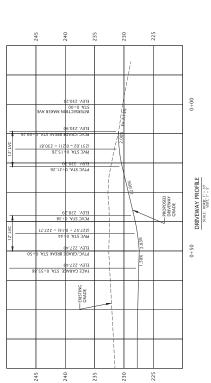


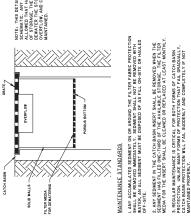
L/ADDEN/2020/3 GEVELOWENTY/CAD/HOST DRAWMS/SWIEW-g-wein-1202880L644g



L/30187450387.3 DEVELOPMENT/CAD/HOST DRAWHSS/Building Power/22038P0Lowg

NW λ_4 , SW λ_4 SECTION 12, TOWNSHIP 24 N, RANGE 4 E, W.M. CITY OF MERCER ISLAND, KING COUNTY, WASHINGTON





3. THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN SHALL BE CLEARLY FLACED IN THE FIELD PRIOR TO ENGINEER THE FLACED CHARMACHINE TO SHALL SHOWN ON DETAINING THE FLACED CLEARING LIMITS TO PERMITTED. THE FLACED CHARMACHINE DY THE APPLICARY CONTINUENCE TO FIRST FLACED CHARMACHINE OF CONSTRUCTION. PERMITTED. THE FLACED CHARMACHINE DY THE APPLICARY CONTINUENCE TO THE PUBLICARY OF CONSTRUCTION. THE IMPERENTATION OF THESE ESC PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND INCRADING OF THESE ESC PACILITIES IN THE STRONGBILLY OF HE PIPILOLANT CONTINACTOR UNTIL ALL CONSTRUCTION IS COMPLETED AND APPROVED AND VEGETAL IDDIVIDATES OF STREAMSHIP. APPROVAL OF THIS EROSION, SEDMENTATION CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAWGE DESIGN (E.C. SIZE AND LOCATION OF ROADS, PIPES, RESTRICTIONS, CHANNESS, RETEXTION FAGILITES, UTILITIES, ETC).

NOTE: THIS DETAIL IS ONLY SCHEMING, ANY INSERT IS ALLOWED THAT HAS A MIN, OS C.F. OF STORMER, THE MINNS TO DE WATER THE STORED SEDIMENT, AN OVERFLOW, AND CAN BE EASILY MANITAINED.

THE EX; FACILITIES SHOWN ON THIS RAM ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE EXCY CANDITIONS SHOULD BE FORGADED AS MEDBEFOR UNEXPECTED STORM EVENTS AND THE CHARGADED AS MEDBEFOR UNEXPECTED STORM EVENTS AND TO ROUSE THAT STEEP THE SITE. 4. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN CONJUNCTION WITH ALL CLEARING AND GRADING ACTIVITIES, AND IN SUCH A MANNERS AST DIVISIEST ENTAI SEDBINEM FAND SEDIMENT LADEN WATER DO NOT ENTER THE DRAINAGE SYSTEM, ROADMAYS, OR VIOLATE APPLICABLE WATER STANDARDS.

A DONTS IN FILTER FABRIC SHALL BE SPLICED AT POSTS USE STAPLES, WIRE RNGS, OR EQUIVALENT TO ATTACH FABRIC TO POSTS

CATCH BASIN INSERT DETAIL

BACKFILL TRENCH WITH NATIVE SOL OR 3/4 1,5 WASHED GRAVEL 2"x 4" WOOD POSTS STEEL FENCE POSTS, REBAR, OR EQUIVALEN

STABILZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DEGLIACY ADDITIONAL ADDITIONAL THE MOSTLAND NOT THE PROPER ADDITIONAL THE MOSTLAND AND THE PROPER ARE KEPT LICEAN FOR THE DISTRICTION OF THE PROPER.

2"12"x14 GA WIRE OR EQUIVALENT, IF STANDARD STRENGTH FABRIC USED —

THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH OR WITHIN THE 48 HOURS FOLLOWING A MAJOR STORM EVENT: B. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDMENT BE ALLOWED TO ACCUMULATE WITHIN A TRAPPED CATCH BASIN. ALL CATCH BASIN AND CONVEYABLE INES SHALL BE CLEANING TO PAVING. THE CLEANING OPERATION SHALL NOT FLUSH SEDMENT LADEN WATER INTO THE DOWNSTREAM SYSTEM.

S. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/CONTRACTOR AND MAINTAINED AS NECESSARY TO ENSURE THEIR CONTINUED FUNCTIONING.

NOTE: FILTER FABRIC FENCES SHALL BE INSTALLED ALONS CONTOUR WHENEVER POSSIBLE,

MANTHORITONISTIC TROUBERS

I THE THE STATE OF THE STATE OF THE THE THE THE STATE OF THE STATE OF

SILT FENCE



STRAND PROPERTY 6950 SE MAKER ST., CITY OF MERCER ISLAND R **DETAILS AND NOTES**

C-3

Know what's below. Call before you d

00318

GOLDSMITH

LAND BEVELOPMENT SERVICES

Strand Property 6950 SE Maker Street, Mercer Island SF Residence Building Permit

Storm Drainage Report



July 2022 Job Number: 22038



SECTION 1: PROJECT OVERVIEW

Project Location

The project site is an 8,750 sf single-family lot located at 6950 SE Maker St, Mercer Island (Tax Parcel #935090-0620). The project site is located in the northwest quarter, of the southwest quarter of Section 12, Township 24 North, Range 4E, W.M. See the attached Vicinity Map (Figure 1) and Aerial photo (Figure 2) for the project location.

Project Description

The project will remove the existing residence and a new single-family residence will be constructed. Proposed improvements will include utilities; driveway improvements for access from SE Maker St; site grading for the new single-family residence, and a connection to the City of Mercer Island stormwater system. Refer to Figure 3 for the project site plan.

SECTION 2: EXISTING CONDITIONS

The project site currently contains one existing home as shown in the photo below. Access to the site is provided directly by SE Maker St, the road directly fronting the project to the south. The home is located in a residential neighborhood with existing single-family homes directly to the north, east, west, and across the street to the south, as shown in the attached aerial photo (Figure 2).



Existing Project Site



Site topography is shown on the topographic survey included in Figure 4. The existing house is located over the eastern area of the lot extending towards the north boundary. Driveway access is near the east boundary and steeply ascends from SE Maker St at a relatively steep grade of approximately 20%. The yard area adjacent to the house to the west and north of the house is relatively flat (5% to 10%) trending in a westerly direction. This area slopes to an approximate elevation of 230.0 ft before becoming steeper (~20%) and sloping towards a tiered rockery along the west site boundary. This rockery wraps around to the front (south) property boundary with the height tapering off as it continues to the east. This rockery along the west boundary is a shorter 2 to 3 ft high upper rockery located above a 9 to 10 ft higher rockery.

The southeast (front) portion of the lot site is predominately covered with driveway area the areas to the west and north of the existing house are landscaped with a mix of lawn area and trees/ shrubs. A large portion of the rear yard to the north is covered by existing decks and patio areas. **Under the existing condition, 5,759 sf (66%) of the existing lot area is covered by impervious/ hardscape areas.** This impervious area consists of a roof area (3,042 sf), driveway (1,024 sf), miscellaneous concrete walks and patios, and deck and wall areas. The existing site topography and surface features are shown in Figure 4.

Site Soils Conditions

Soils for the site have been mapped as Arents, Alderwood material (AmC). These soils are classified as SCS Hydrologic Group C till soils. A map generated from the NRSC Soils Web showing soil types in the vicinity of the project site has been included in Figure 5. Investigations by the project geotechnical engineer indicate consistency with glacial till soils however, soils observed within the west yard area were primarily loose, silty sand fill. The site is mapped as infiltration infeasible as shown on Figure 7. Groundwater was not observed during site investigations. Please refer to the project geotechnical report or additional information regarding the site soil conditions and an assessment of the site's critical areas. A copy of the City GIS mapping for steep slope and erosion hazard areas has been attached as Figure 8.

Existing Site Drainage Patterns

Under the existing condition, roof runoff is collected and directed to a roof drain system connecting to the public storm drain system within the SE Maker St right-of-way. Yard runoff from the rear and west lot areas sheet flows to the west towards the previously described rockery and ultimately onto the adjacent property. The driveway sheet flows onto the SE Maker St roadway before entering the system downstream of the property to the west.

Section 3.0 Downstream Analysis

Site runoff enters the city storm system, a public closed conveyance system draining to the west along SE Maker St reaching Lake Washington approximately 1,000 ft downstream of the site. Elements of the storm system are shown in the City GIS mapping (Figure 6) and as described in detail below. Based on on-site observations, and the slope of the system there appears to be no downstream constraints. On the day of observation (07/08/2022), the city was relining the last segment downstream from SE Mercer Way to Lake Washington.

Also, the amount of impervious area from the site is reduced under the developed condition reducing the amount of runoff directed to the downstream system (see Section 4). Based on the downstream system, and the reduction in site runoff, detention is not required for this site.



The following is a description of the existing stormwater flow path, See Figure 6 for more detail, references to the location identifiers are included below:

- Runoff from upstream areas is directed into the public storm system in SE Maker St. (Photo 1).
- 2. Runoff is discharged from the site to the storm system within SE Maker St. right-of-way. The system is a 12-inch pipe that runs west within SE Maker St., **Photo 2**, **3**, **and 4**.
- 3. Flow travels west steeply towards the west portion of SE Maker St. 165 ft downstream of the site, **Photos 5.**
- 4. Stormwater continues westward along SE Maker St. within a 12-inch pipe to SE Mercer Way 450 ft downstream of the site. **Photo 6-7**
- 5. Runoff continues north along SE Mercer Way through an 18-inch pipe system for approximately 160 feet. **Photo 8**
- 6. Runoff travels west towards Lake Washington through a 12" pipe system 1,000 ft downstream of the site. This system has just been relined. **Photo 9-10**
- 7. System discharges to Lake Washington.



Photo 1 is looking west from the intersection of SE Maker St and 72nd Ave SE. This catch basin is where drainage from 72nd Ave SE storm system turns westward towards Lake Washington via SE Maker St.

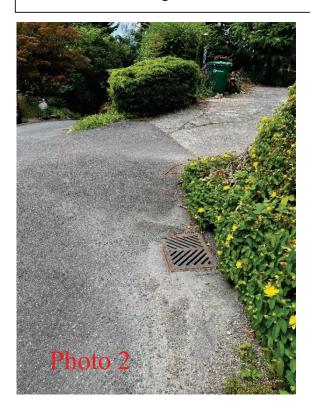


Photo 3 shows the existing catch basin at the middle of the property. This is the structure that will receive the proposed connection from the site. The 12" line has a slope of approx.



Photo 2 is looking at the catch basin located at the east property line. Drainage system is 12" in diameter with an approx. slope of 15%.

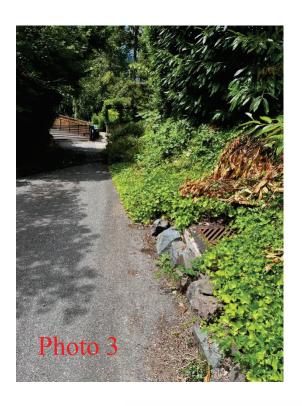




Photo 5 shows the continuation of the upper portion of SE Maker St. The photo shows the last catch basin before the system descends steeply towards the western portion of SE Maker. The pipe is 12" in diameter and has a slope of 40% through the piped section west of this structure.



Photo 4 shows the existing catch basin at the southwest corner of the site on SE Maker St.



Photo 6 shows the western portion of SE Maker St towards SE Mercer Way. The storm system is 12" in diameter and has a slope of approximately 15 to 25%.





The storm system along SE Maker collects street drainage. Photo 7 shows the pair of catch basins located directly upstream of SE Mercer Way.

Photo 8 shows the structure redirecting the SE Maker system north along SE Mercer Way. The system is relatively flat and has an 18" diameter. GIS mapping shows a surface drainage feature draining to this location. There are no signs that this feature exists.





Photo 9 shows the structure upstream of where the system is redirected to Lake Washington.



Photo 10 is looking at the west side of SE Mercer Way. The city is relining the existing system towards the Lake.



Section 4 Conveyance Analysis

The potential impacts on the conveyance capacity of the stormwater conveyance system downstream of the project have been assessed. Site drainage is directed to a stormwater conveyance system within SE Maker St as previously described. Peak stormwater runoff rates were calculated using WWHM2012 with a 15 min time step. The simulated developed flow rate decreased by 0.0084 cfs relative to the existing condition. A copy of the simulation results and basin area assumptions are included herein. This analysis assumes that the predeveloped conditions are that of the existing home, not historical conditions.

Peak Flow: Ex. Q₁₀₀=0.1322 cfs Dev Q₁₀₀=0.1238 cfs

Existing Site Conditions					
	SF	AC			
Total Site Area (Target Pervious and Impervious Area)	8,750	0.201			
Roof / Shed	3,042	0.070			
Conc. Driveway	1,024	0.024			
Conc Walk	200	0.005			
Pavers	542	0.012			
Deck	117	0.003			
Wood Steps and Walls	40	0.001			
Block Wall	4	0.000			
Rockery	790	0.018			
On-Site Impervious	5,759	0.132			
On-Site Pervious	2,991	0.069			
Off-Site Impervious	333	0.008			
Off-Site PerviousImpervious	150	0.003			
Total Basin Area	9,233	0.212			
Total Model Areas					
Effective Impervious Area	6,092	0.140			
Landscape (Till Grass)	2,991	0.069			
Total Project Area	9,233	0.212			

Developed Site Conditions						
	SF	AC				
Total Site Area (Target Pervious and Impervious Area)	8,750	0.201				
Roof	1,808	0.042				
Stairs	78	0.002				
Retaining Wall	20					
Deck	91					
Ex. Wood Wall	5					
Ex. Rockery	731					
Conc Driveway	804	0.018				
On-Site Impervious	3,537	0.081				
On-Site Pervious	5,213	0.120				
Off-Site Impervious	483	0.011				
Total Basin Area	9,233	0.212				
Total Model Areas						
Effective Impervious Area	4,020	0.092				
Landscape (Till Grass)	5,213	0.120				
Total Project Area	9,233	0.212				

WWHM2012 PROJECT REPORT

Project Name: 22038

Site Name: Site Address: City :

Report Date: 7/10/2022
Gage : Seatac

Data Start : 1948/10/01
Data End : 2009/09/30
Precip Scale: 1.00
Version Date: 2019/09/13

Version : 4.2.17

Low Flow Threshold for POC 1 : 50 Percent of the 2 Year

High Flow Threshold for POC 1: 50 year

PREDEVELOPED LAND USE

Name : Existing

Bypass: No



GroundWater: No

Pervious Land Use acre
C, Lawn, Mod .069

Pervious Total 0.069

Impervious Land UseacreROADS FLAT0.143

Impervious Total 0.143

Basin Total 0.212

Element Flows To:

Surface Interflow Groundwater

MITIGATED LAND USE

Name : Developed

Bypass: No

 $\textbf{GroundWater:} \ \ \texttt{N} \texttt{o}$

Pervious Land Use acre
C, Lawn, Mod .092

Pervious Total 0.092

Impervious Land Use acre
ROADS FLAT 0.12

Impervious Total 0.12

Basin Total 0.212

Element Flows To:

Surface Interflow Groundwater

ANALYSIS RESULTS

Stream Protection Duration



Predeveloped Landuse Totals for POC #1

Total Pervious Area:0.069
Total Impervious Area:0.143

Mitigated Landuse Totals for POC #1

Total Pervious Area:0.092 Total Impervious Area:0.12

Flow Frequency Return Periods for Predeveloped. POC #1

Return Period	Flow(cfs)
2 year	0.059797
5 year	0.077646
10 year	0.090033
25 year	0.106372
50 year	0.119062
100 year	0.13221

Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs)
2 year	0.052975
5 year	0.070058
10 year	0.082082
25 year	0.098118
50 year	0.110694
100 year	0.123824

Stream Protection Duration

Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1949	0.083	0.076
1950	0.078	0.068
1951	0.051	0.046
1952	0.040	0.034
1953	0.043	0.037
1954	0.049	0.043
1955	0.054	0.048
1956	0.053	0.047
1957	0.064	0.058
1958	0.048	0.042
1959	0.046	0.039
1960	0.053	0.048
1961	0.053	0.047
1962	0.044	0.037
1963	0.052	0.047
1964	0.048	0.042
1965	0.066	0.060
1966	0.041	0.036
1967	0.073	0.068
1968	0.084	0.074
1969	0.060	0.054
1970	0.056	0.050
1971	0.067	0.060



J	•		
1972 1973 1974	0.075 0.038 0.063	0.069 0.032 0.057	
1974 1975 1976	0.065 0.048	0.057	
1977 1978 1979	0.047 0.059 0.079	0.041 0.051 0.067	
1980 1981	0.085 0.062	0.079 0.054	
1982 1983 1984	0.091 0.068 0.045	0.083 0.059 0.040	
1985 1986	0.062 0.052	0.054	
1987 1988 1989	0.079 0.046 0.058	0.067 0.039 0.048	
1990 1991 1992	0.126 0.096 0.045	0.120 0.090 0.039	
1993 1994	0.037 0.038	0.032 0.032	
1995 1996 1997	0.054 0.064 0.060	0.048 0.059 0.054	
1998 1999	0.056 0.126	0.048 0.114	
2000 2001 2002	0.060 0.061 0.081	0.054 0.052 0.073	
2003 2004 2005	0.064 0.116 0.053	0.059 0.105 0.047	
2006 2007 2008	0.048 0.113 0.094	0.043 0.108 0.088	
2009	0.071	0.059	

Stream Protection Duration

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	Mitigated
1	0.1259	0.1196
2	0.1257	0.1144
3	0.1163	0.1076
4	0.1125	0.1050
5	0.0963	0.0900
6	0.0941	0.0877
7	0.0908	0.0829
8	0.0847	0.0787
9	0.0837	0.0763
10	0.0834	0.0741
11	0.0806	0.0734
12	0.0793	0.0688
13	0.0787	0.0684
14	0.0784	0.0681



15	0.0748	0.0674	
16	0.0730	0.0673	
17	0.0707	0.0600	
18	0.0684	0.0598	
19	0.0673	0.0594	
20	0.0664	0.0590	
21	0.0648	0.0589	
22	0.0641	0.0589	
23	0.0641	0.0587	
24	0.0641	0.0577	
25	0.0633	0.0571	
26	0.0619	0.0545	
27	0.0618	0.0544	
28	0.0610	0.0544	
20 29			
	0.0603 0.0603	0.0543 0.0538	
30			
31	0.0602	0.0520	
32	0.0586	0.0508	
33	0.0577	0.0498	
34	0.0562	0.0484	
35	0.0556	0.0481	
36	0.0545	0.0480	
37	0.0545	0.0478	
38	0.0535	0.0476	
39	0.0533	0.0474	
40	0.0529	0.0472	
41	0.0526	0.0468	
42	0.0523	0.0468	
43	0.0518	0.0467	
44	0.0511	0.0464	
45	0.0490	0.0435	
46	0.0485	0.0434	
47	0.0483	0.0433	
48	0.0481	0.0423	
49	0.0476	0.0421	
50	0.0465	0.0411	
51	0.0462	0.0395	
52	0.0461	0.0393	
53	0.0449	0.0388	
54	0.0446	0.0387	
55	0.0435	0.0374	
56	0.0434	0.0369	
57	0.0414	0.0361	
58	0.0401	0.0340	
59	0.0385	0.0323	
60	0.0379	0.0321	
61	0.0372	0.0318	

Stream Protection Duration POC #1
The Facility PASSED

The Facility PASSED.

Flow(cfs) Predev Mit Percentage Pass/Fail

0.0299 1619 978 60 Pass



0 0200	1 / E O	000	C 0	Daga
0.0308	1459	882	60	Pass
0.0317	1295	786	60	Pass
0.0326	1170	719	61	Pass
0.0335	1051	655	62	Pass
0.0344	963	588	61	Pass
0.0353	879	542	61	Pass
0.0362	793	497	62	Pass
0.0371	727	453	62	Pass
0.0380	667	417	62	Pass
0.0389	615	381	61	Pass
0.0398	568	351	61	Pass
0.0407	527	336	63	Pass
0.0416	489	313	64	Pass
0.0425	443	293	66	Pass
0.0434	409	266	65	Pass
0.0443	382	242	63	Pass
0.0452	358	226	63	Pass
0.0461	342	213	62	Pass
0.0470	317	193	60	Pass
0.0479	294	175	59	Pass
0.0488	270	161	59	Pass
0.0497	251	152	60	Pass
0.0506	227	142	62	Pass
0.0515	214	129	60	Pass
0.0524	201	121	60	Pass
0.0524	190	116	61	
		110		Pass
0.0542	174		63	Pass
0.0551	164	104	63	Pass
0.0560	155	101	65	Pass
0.0569	144	94	65	Pass
0.0578	134	89	66	Pass
0.0587	125	84	67	Pass
0.0596	117	75	64	Pass
0.0605	107	71	66	Pass
0.0614	104	70	67	Pass
0.0623	99	69	69	Pass
0.0632	96	64	66	Pass
0.0641	86	60	69	Pass
0.0650	82	58	70	Pass
0.0659	81	56	69	Pass
0.0668	78	52	66	Pass
0.0677	74	45	60	Pass
0.0686	72	41	56	Pass
0.0695	64	38	59	Pass
0.0704	64	36	56	Pass
0.0713	61	34	55	Pass
0.0722	57	32	56	Pass
0.0731	53	31	58	Pass
0.0740	52	30	57	Pass
0.0749	49	29	59	Pass
0.0758	42	24	57	Pass
0.0767	41	23	56	Pass
0.0776	39	23	58	Pass
0.0785	36	22	61	Pass
0.0794	33	19	57	Pass
0.0803	32	19	59	Pass
0.0812	29	19	65	Pass
		_ ~		_



	3				
0.0821	29	18	62	Pagg	
0.0830	28	16	57	Pass Pass	
0.0839	23	15	65	Pass	
0.0848	21	14	66	Pass	
0.0857	20	13	65	Pass	
0.0857	20	13	65	Pass	
0.0875	19	13	68	Pass	
0.0873	19	12	63	Pass	
0.0893	16	10	62	Pass	
0.0093	16	9	56		
0.0902			57	Pass	
	14	8 7	53	Pass	
0.0920 0.0929	13	7	53	Pass	
	13			Pass	
0.0938	13	7 7	53	Pass	
0.0947	11		63	Pass	
0.0956	10	7	70	Pass	
0.0965	8	7	87	Pass	
0.0974	8	7	87	Pass	
0.0983	8	6	75 75	Pass	
0.0992	8	6	75	Pass	
0.1001	8	5	62	Pass	
0.1010	8	5	62	Pass	
0.1019	7	4	57	Pass	
0.1029	7	4	57	Pass	
0.1038	7	4	57	Pass	
0.1047	7	4	57	Pass	
0.1056	7	3	42	Pass	
0.1065	7	3	42	Pass	
0.1074	7	3	42	Pass	
0.1083	7	2	28	Pass	
0.1092	6	2	33	Pass	
0.1101	6	2	33	Pass	
0.1110	5	2	40	Pass	
0.1119	5	2	40	Pass	
0.1128	3	2	66	Pass	
0.1137	3	2	66	Pass	
0.1146	3	1	33	Pass	
0.1155	3	1	33	Pass	
0.1164	3	1	33	Pass	
0.1173	2	1	50	Pass	
0.1182	2	1	50	Pass	
0.1191	2	1	50	Pass	

Water Quality BMP Flow and Volume for POC #1

On-line facility volume: 0 acre-feet On-line facility target flow: 0 cfs.

Adjusted for 15 min: 0 cfs.

Off-line facility target flow: 0 cfs.

Adjusted for 15 min: 0 cfs.

LID Report

LID Technique Used for Total Volume Volume Infiltration Cumulative Percent Water Quality Percent Comment



Water Ovality	Treatment?	Needs	Through	Volume	Volume	Volume
Water Quality		Treatment	Facility	(ac-ft.)	Infiltrat	cion
Infiltrated	Treated	(5.1)	(5.1)		G 11.	
		(ac-ft)	(ac-ft)		Credit	
Total Volume Infiltrated		0.00	0.00	0.00		0.00
0.00 0%	No Treat. Cr	redit				
Compliance with LID Standa	ard 8					
Duration Analysis Result =	= Passed					

Perlnd and Implnd Changes

No changes have been made.

This program and accompanying documentation is provided 'as-is' without warranty of any kind. The entire risk regarding the performance and results of this program is assumed by the user. Clear Creek Solutions, Inc. disclaims all warranties, either expressed or implied, including but not limited to implied warranties of program and accompanying documentation. In no event shall Clear Creek Solutions, Inc. be liable for any damages whatsoever (including without limitation to damages for loss of business profits, loss of business information, business interruption, and the like) arising out of the use of, or inability to use this program even if Clear Creek Solutions, Inc. has been advised of the possibility of such damages.

